GENERAL REQUIREMENTS:

1. PRIOR TO STARTING ANY WORK THE CONTRACTOR SHALL REVIEW THESE PLANS AND SITE CONDITIONS AND NOTIFY THE ENGINEER IF ANY DISCREPANCIES ARE DISCOVERED. 2. THE ENGINEER IS NOT RESPONSIBLE FOR THE SUPERVISION OF THE CONTRACTOR NOR HIS EMPLOYEES DURING THE CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE MEANS AND ESTABLISH METHODS OF THE CONSTRUCTION TO MEET REQUIREMENTS OF ALL APPLICABLE CODES, INDUSTRY STANDARDS AND REQUIREMENTS OF THESE PLANS. 3. QUALITY OF THE WORK SHALL MEET OR EXCEED INDUSTRY STANDARD PRACTICES. 4. ANY DEVIATIONS FROM THESE PLANS SHALL BE REVIEWED AND APPROVED BY THE ENGINEER.

SUBMITTALS:

1. THE CONTRACTOR SHALL PROVIDE 5 ORIGINAL SUBMITTALS FOR ALL MATERIALS, PRODUCTS, AND ACCESSORIES USED FOR THIS PROJECT (NO EXCEPTIONS). 2. All submittals shall be reviewed, approved, dated and signed by the General

CONTRACTOR REPRESENTATIVE PRIOR SUBMITTING FOR ENGINEERS REVIEW. HARDWARE 3. THE CONTRACTOR SHALL PREPARE AND MAINTAIN SUBMITTAL LOG FOR ALL PRODUCT WITH 1. HARDWARE SHALL BE 316 STAINLESS STEEL OR BETTER OR ZMAX GALVANIZED FOR NON LIST OF ALL SUBMITTALS, THEIR STATUS AND DATES OF SUBMITTAL AND APPROVAL. EXPOSED SIMPSON PRODUCTS, UNLESS OTHERWISE SPECIFIED.

4. THE CONTRACTOR SHALL PLAN IN ADVANCE FOR TIMELY PREPARATION OF ALL SUBMITTALS AND ALLOW 2-3 WEEKS FOR REVIEW AND CORRECTION PROCESS.

DESIGN DATA:

1. APPLICABLE BUILDING CODE: FBC 6TH EDITION (2017)

- 2. APPLICABLE DESIGN LOADS: PER ASCI/SEI 7-10
- FLOOR LIVE LOAD 40 PSF
- ROOF LIVE LOAD: 20 PSF (300 LB CONC.) BASIC WIND SPEED: 180 MPH
- Exposure: D
- STRUCTURAL CATEGORY: II
- FLOOD ZONE: AE8

FLOOD RESISTANT DESIGN AND CONSTRUCTION PER ASCI 24-14

FLOOD DESIGN CLASS 2, WET FLOOD PROOFING

ALL PRESSURES SHOWN ARE BASED ON ASD DESIGN WITH A LOAD FACTOR OF 0.6

Soils and Foundations:

FOUNDATIONS SHALL BE PLACED ON A "SEDIMENTARY AND FOLIATED ROCK" WITH A ALLOWABLE LOAD BEARING PRESSURE OF 3,000 PSF. NOTIFY THE ENGINEER OF SOIL CONDITIONS ARE DIFFERENT.

1. ALL FOUNDATIONS, SLABS AND FOOTERS SHALL BE PLACED ON STABILIZED UNDISTURBED SUBGRADE SOIL.

- 2. MINIMUM FOUNDATION DEPTH SHALL BE 24" UNLESS OTHERWISE IS SPECIFIED ON THE PLANS. IF OVER-EXCAVATED - FILL SHALL NOT BE PLACED BACK INTO THE TRENCH UNLESS APPROVED BY THE ENGINEER.
- 3. FILL UNDER THE FOUNDATIONS SHALL BE USED ONLY IF APPROVED BY THE ENGINEER. CLEAN FILL MATERIAL SHALL BE PLACED IN 6"-8" LAYERS AND COMPACTED TO 98% DENSITY USING THE MODIFIED PROCTOR TEST.
- 4. FILL MATERIAL SHALL BE CLEAN GRANULAR SAND OR LIMEROCK MIX WITHOUT ANY ORGANIC MATERIALS, CLAY, MUCK AND ROCKS LARGER THAN 4". BACKFILL SHALL NOT CONTAIN ANY WOOD OR CELLULOSE DEBRIS.

AUGERCAST PILES

- 1. AUGERCAST PILES SHALL BE 16" DIAMETER WITH MINIMUM EMBEDMENT OF 3FT INTO THE CAP ROCK UNLESS OTHERWISE SHOWN ON THE PLANS.
- 2. CONCRETE FOR PILES SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 5000 PSI. WATER CEMENT RATIO SHALL NOT EXCEED W/C=0.40.
- 3. REINFORCEMENT SHALL BE FOUR (4) #6 REBAR VERTICALLY WITH #3 HOOPS AT 10' O.C. CONTRACTOR SHALL USE PLASTIC CHARS OR CENTRALIZERS TO PROVIDE A 3" COVER ON ALL SIDES OF THE REINFORCEMENT.

Concrete

1. APPLICABLE CODE ACI 318 LATEST EDITION AND ACI 301.

- 2. ALL CONCRETE ELEMENTS SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 4000 PSI UNLESS OTHERWISE IS SHOWN ON THE PLANS. WATER CEMENT RATIO SHALL NOT EXCEED W/C = 0.40.
- 3. All CAST-IN-PLACE CONCRETE SHALL BE CURED AND PROTECTED FROM OVERDRYING PER ACI 305R-10 "Hot Weather Concreting".
- 4. All exposed edges shall have 1/2 chamfers.
- 5. NO COLD JOINTS ARE ALLOWED UNLESS OTHERWISE APPROVED BY THE ENGINEER.
- 6. TESTING: ALL FIELD AND LABORATORY TESTING SHALL BE PERFORMED BY THE INDEPENDENT SPECIALIZED COMPANY. THE CONTRACTOR IS RESPONSIBLE FOR ALL SCHEDULING, COORDINATION AND COST OF
- TESTING COMPANY.

THREE (3) SAMPLES SHALL BE TAKEN AND TESTED EACH TIME.

- MINIMUM SAMPLING FREQUENCY:
- A) EACH DAY OF CONCRETING FOR EVERY CONCRETE MIX;
- B) EVERY 50 CUBIC YARDS;
- C) EVERY 2000 SQ.FT. OF SLAB AREA.

ALL TESTING SHALL BE PER LATEST ACI AND ASTM REQUIREMENTS.

LABORATORY SHALL SUPPLY THREE (3) ORIGINAL SIGNED&SEALED REPORT RESULTS TO THE ENGINEER.

7. CAST-IN-PLACE AND PRECAST MEMBERS ERECTION TOLERANCES SHALL BE AS SPECIFIED IN TABLE 8.2.2 OR IN SECTION 8.3 OF "PCI DESIGN HANDBOOK/SIXTH EDITION".

Reinforcement

- 1. ALL REBAR SHALL BE ASTM A1035 GRADE 100 (CHROMX 9100) AS CORROSION RESISTANT REINFORCEMENT.
- 2. All requirements for placement, cover, tolerances, etc. Shall be per ACI
- 318-11. 3. ALL HOOKS AND BENDS SHALL BE FACTORY MADE UNLESS FIELD BENDS ARE APPROVED
- BY THE ENGINEER.
- 4. ONLY PLASTIC CHAIRS AND CENTRALIZERS SHALL BE USED FOR REBAR SUPPORT.

STRUCTURAL LUMBER

- 1. ALL WOOD MEMBERS SHALL MEET OR EXCEED REQUIREMENTS SPECIFIED IN "ANSI/AF&PA NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION" AND ALL REFERENCED STANDARDS.
- 2. All wood members shall be Souther Pine No2 or Greater kiln dried as specified in THE STANDARDS, UNLESS OTHERWISE SPECIFIED.
- 3. All wood members exposed to exterior, in direct contact with concrete or steel SHALL BE PRESSURE-TREATED (PT) UC4B GRADE PER AWPA STANDARDS.
- 4. ALL FIELD CUTS IN PT LUMBER SHALL TREATED ON SITE. 5. NAILING SHALL BE IN ACCORDANCE WITH FBC 2017. NAILS AND OTHER FASTENERS FOR PT WOOD SHALL BE STAINLESS STEEL OR ACQ APPROVED TREATED.
- 6. Sheathing shall be 5/8" CDX Plywood Sheathing Grade, unless otherwise is SPECIFIED ON THE PLANS. USE 10D RING-SHANK NAILS WITH SPACING OF 4" O.C. ON ALL EDGES AND 6" O.C. IN THE FIELD.

2. ALL CONNECTORS SHALL HAVE STAINLESS STEEL SCREWS AND FASTENERS OR ACQ APPROVED TREATED (FOR NON EXPOSED LOCATIONS).

REINFORCED MASONRY (CMU)

1. All Masonry shall be reinforced concrete masonry unit in accordance with the LATEST EDITION OF ACI 530/ASCE 5/TMS 402.

2. INSTALL ALL BLOCKS IN RUNNING BOND.

3. MINIMUM MASONRY BLOCK (ASTM C90) STRENGTH SHALL (F'M) BE 1500 PSI.

4. Type "S" mortar (ASTM C270) shall be used using 3/8" full bedding reinforced w/ 9 GAGE 304 STAINLESS STEEL LADDER WIRE EVERY 2ND ROW.

5. FILLED CELLS SHALL BE REINFORCED WITH #5 REBAR @ 24" D.C. (UNLESS OTHERWISE IS SPECIFIED ON THE PLANS).

6. GROUT SHALL BE PEA ROCK PUMP MIX (ASTM C476) WITH A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI (28 DAY) (ASTM C1019). TARGETED SLUMP SHALL BE 8"-11". 6. EACH GROUTED CELL SHALL HAVE CLEANOUT OPENINGS AT THE BOTTOM. THERE SHALL BE NO LOOSE MORTAR OR OTHER DEBRIS IN THE BOTTOM OF THE CELL. USE BLAST PRESSURE WASHING FOR SURFACE PREPARATION.

STRUCTURAL STEEL

1. STRUCTURAL STEEL COMPONENTS SHALL BE AS DESCRIBED IN "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" AISC 2005 OR LATER EDDITION.

- 2. HSS SHAPES (STRUCTURAL TUBING) SHALL BE ASTM A500 (FY=46 KSI).
- 3. STEEL PLATES, FLANGES AND MISCELENIOUS ELEMENTS SHALL BE ASTM A36 (FY=36 KSI) UNLESS NOTED OTHERWISE ON THE PLANS.
- 4. W-SHAPES, C-SHAPES AND OTHER FORMED STEEL SHALL BE ASTM A36 (FY=36 KSI). 5. ALL WELDING SHALL BE IN CONFORMANCE WITH THE LATEST SPECIFICATIONS AWS
- D1.1/D1.1M:2017, STRUCTURAL WELDING CODE STEEL.

6. BOLTS: HOT DIP GALVANIZED. A325N, A563DH HEX NUTS, F436 WASHERS.

7. ANCHOR BOLTS: HOT DIP GALVANIZED. A307 GRADE A, A563DH HEX NUTS, F844 WASHERS.

STRUCTURAL STEEL COATING

1. ALL SURFACES SHALL BE ABRASIVE BLAST CLEANED TO NEAR-WHITE METAL (PER SSPC-SP10)

EXPOSED STEEL:

2. All surfaces shall be primed with Polyamide Epoxy - one coat (8.0 mils DFT).

- 3. APPLY SEALANT AT ALL LOCATIONS WHERE STEEL IS WELDED, LAPPED ETC. SEALANT
- MATERIAL SHALL BE COMPATIBLE WITH THE PAINTING SYSTEM.
- 4. TOP LAYER SHALL BE TWO (2) COAT POLYURETHANE (3.0 MILS DFT EACH).
- 5. TOP PAINT SHALL BE UV RESISTANT OR HAVE A UV RESISTANT COATING.
- 6. COLORS SHALL MATCH EXISTING OR TO BE SELECTED BY THE OWNER.
- Non-Exposed Steel (Interior):

7. 2 COATS OF "SUMTER COATINGS" UNIVERSAL PRIMER (6.0 MILS DFT) OR APPROVED EQUAL.

ALUMINUM COMPONENTS

1. TYPE 6061-T6 ALUMINUM.

2. MIG WELDED ALL JOINTS W/ CONTINUOUS 1/8" WELD. USE 5356 FILLER WIRE ALLOY.

3. ALL ALUMINUM IN CONTACT WITH CONCRETE, PT WOOD, DISSIMILAR

METALS AND OTHER CORROSIVE MATERIALS SHALL COATED WITH

COAL-TAR EPOXY OR PROTECTED BY OTHER ENGINEER APPROVED

METHOD.

OPENINGS: 1. All exterior windows & doors shall be large and small missile impact

RATED OR HAVE CODE COMPLIANT SHUTTERS. 2. All exterior windows and doors shall have Florida product approval

AND NOA. PRODUCT APPROVAL LABELS SHALL BE PERMANENTLY ATTACHED TO THE FRAME.

3. WIND PRESSURE ON COMPONENTS AND CLADDING (CH 30 PART 1)

Nominal Width Type and # of opening



- MAX PRESSURE (PSF) MIN PRESSURE (PSF)

Nominal Height

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	Э	2	Э	Э	2	Э
3'-0" TYP. 7			2		1	
	Э	2	Э	Э	2	Э

ROOF WIND PRESSURES DIAGRAM SCALE: NTS

ENCLOSED - BUILDING						
Wind Pressure on Components and Cladding (Ch 30 Part 1)						
DESCRIPTION	Width, ft	Span, ft	Area, ft2	MAX P, PSF	MIN P, PSF	
ZONE 1	1	1	1	N/A	N/A	
ZONE 2	1	1	1	N/A	N/A	
ZONE 3	1	1	1	N/A	N/A	
ZONE 4	1	1	1	+59.64	-64.70	
ZONE 5	1	1	1	+59.64	-79.86	

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DATE

09-30-19 Structural

REVISIONS

DRAWN BY PRS

PROJECT NUMBER 1914



POXY DOWEL #4 HORIZONTAL REBAR WI 12' EMBEDMENT WISIMPSON SETXP ADHESIVE POXY DOWEL #4 HORIZONTAL REBAR BEDUE THE NEW 12x16' CAST-IN PLACE BEADER BEAM INTO THE EXISTING OLUMP ADHESIVE 2' COVER 2' COVER		ANEL JOII	VT	/				
#4 REBAR LAPPED AND EPDXY DOWELED INTO THE EXISTING W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/12" EMBEDMENT W/14" EMBEDMENT H/14" EXISTING DANATER DA PARITO DANATA AND VARITE BARENTO DANATA W/14" EMBEDMENT W/14" E					AIN.1 ⁻ -0" BEDMENT		LAP LENGTH	
EPDXY DOWEL #4 HORIZONTAL REBAR ABOVE THE NEW 12X10° CAST-IN-PLACE BEADER BEAM INTO THE EXISTING DOLUMN W/12' EMBEDMENT W/SIMPSON SETXP ADHESIVE 2' COVER 2'	#4 REBAR LAPPED AND DOWELED INTO THE E WALL W/12" EMBI W/SIMPSON SET-XP AD	D EPOXY— XISTING _ PANEL EDMENT DHESIVE					 	2" COVER
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	B Y S B EPOXY DOWEL ALL VERTIC, B REBAR INTO THE WALL BELC +							



EXISTING WALL PANEL

THE WALL PANEL JOINTS.

NOTE: ALL NEW OPENINGS ARE TO BE CREATED AWAY FROM

SECTION 1-1

SCALE: 1 1/2"=1'-0"

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SCALE: 1 1/2"=1'-0"

ki	PANEL	JOINT

-NEW INTERIOR PARTITION WALL

(ARCHITECTURAL LEGEND "4")

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SIGNATURE
DATE
SERGE MASHTAKOV PROFESSIONAL ENGINEER STATE OF FLORIDA
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<u>'ER</u>	W/ 5.5" METAL STUD WALL @ 16" D.C. (PROSTUD MODEL NO.: 550s162-43P - 18GA PUNCHED STRUCTURAL STUDS) W/ 5/8" TYPE X GYPSUM BOARD ATTACHED TO METAL STUDS W/ MIN. NO.6 TYPE S DRYWALL SCREWS @ 12" D.C. (18'-4" MAXIMUM HEIGHT FOR 16" D.C. STUDS OR 20'-2" MAXIMUM HEIGHT FOR 12" D.C. STUDS)
	EXISTING WALL REINFORCEMENT IS NOT SHOWN FOR CLARITY (SEE CONSTRUCTION DRAWING AND SHOP DRAWINGS FOR DETAILS - AVAILABLE UPON REQUEST)
	#4 IN-FILL HORIZONTAL REBAR W/12" EMBEDMENT, W/SIMPSON SET-XP ADHESIVE AND W/ 20" LAP LENGTH (CENTER LAP LENGTH TO THE CENTER OF THE INFILL)
Y Y	#4 VERTICAL REBAR W/12" EMBEDMENT W/SIMPSON SET-XP ADHESIVE AT THE EXISTING WALL END AND W/ STANDARD HOOK INTO THE NEW HEADER BEAM
P DRAWINGS)	7 1/4" THICK (VERIFY AND MATCH WITH EXISTING PANEL THICKNESS) CAST-IN-PLACE CONCRETE INFILL 4000 PSI W/#4 HORIZONTAL AND VERTICAL REBAR @12" O.C. MAX CHAMFERS ON THE EXTERIOR SIDE TO MATCH W/ ALL EXISTING CHAMFERS ALL FINISH AND PAINT TO MATCH EXISTING
DOOR SHO	NEW 12"X16" CAST-IN-PLACE CONCRETE HEADER BEAM 4000 PSI W/(2) #6 REBAR TOP AND BOTTOM W/#3 STIRRUPS SPACED AS SHOWN
	±2'-0" FROM JOINT TO EXISTING 3'-2" DOOR EDGE (VARIES - SEE ARCH. DWGS)
SOVER	NEW 8"x12" CAST-IN-PLACE CONCRETE COLUMN 4000 PSI W/(4) #5 REBAR AND W/#3 TIES @ 10" O.C.
	SAW CUT IN STRAIGHT LINES IN THE WALL PANEL TO CREATE THE ROUGH OPENING. DO NOT CUT REBAR IN THE WALL. CREATE 3/4" CHAMFERS TO MATCH EXISTING PANELS STYLE
	EPOXY DOWEL #5 COLUMN REBAR INTO THE WALL BELOW W/ 6" EMBEDMENT W/ SIMPSON SET-XP ADHESIVE (ONLY FOR REBAR OVER WALL BELOW, NOT OVER SLAB)
*	